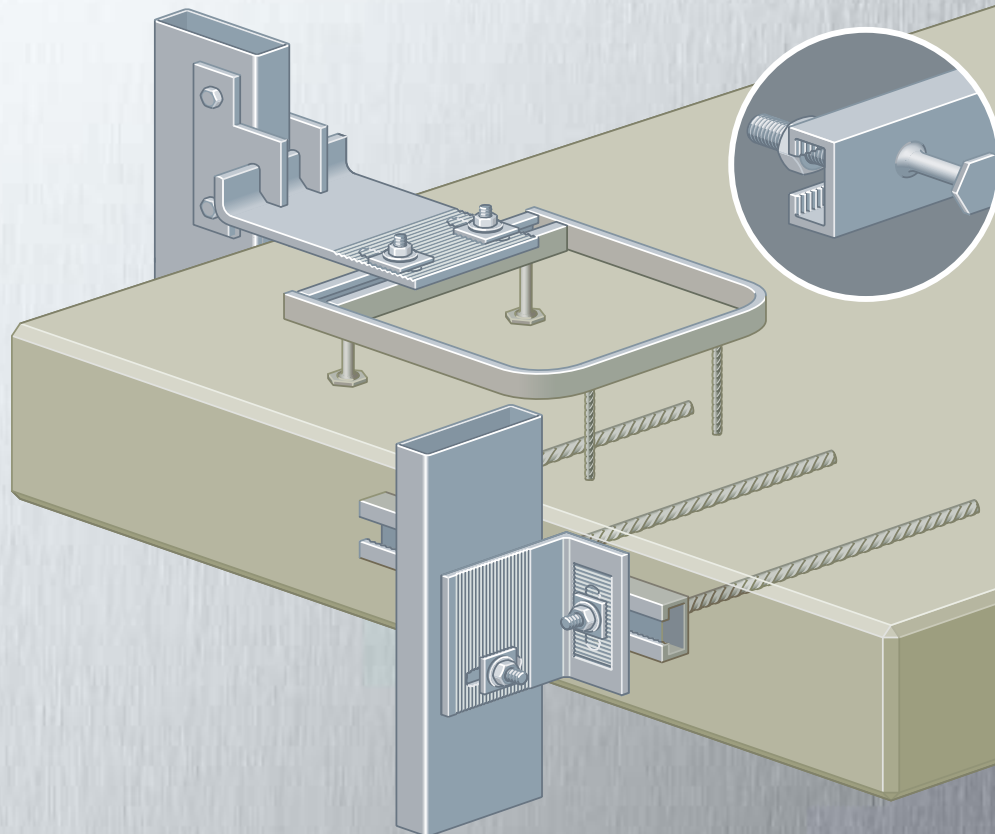


# HALFEN CHANNELS FOR CURTAIN WALL

## PRODUCT INFORMATION



*HALFEN CURTAIN WALL SUPPORT SYSTEM*

HCW 05-CN-E

FACADE



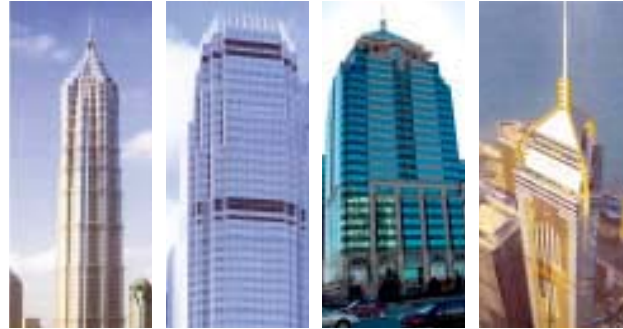
**HALFEN·DEHA**

YOUR BEST CONNECTIONS

# HALFEN-DEHA. WORLDWIDE LEADER IN CURTAIN WALL CONNECTIONS

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Introduction and short project list	2
Installation examples	3
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Halfen channels and T-bolts for standard applications	12-13
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Halfen high load channel HCW 52/34	16-17
Disadvantages of welding and advantages of using Halfen channels	18-19

## SHORT PROJECT LIST



Jin-Mao, Shanghai      IFC One, Hong Kong      Henderson Centre, Beijing      Central Plaza, Hong Kong

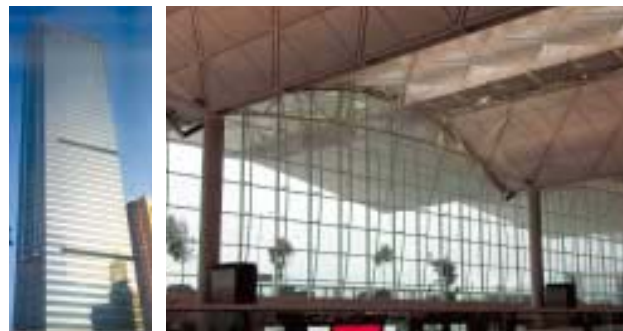
### A Worldwide Reputation for Excellence



In 1929 HALFEN started to produce adjustable connection systems in Germany which soon became known as "Halfen channels". Now used worldwide, the amount of Halfen channels installed in the past 10 years alone would almost span the globe. The product allows adjustable connections to precast concrete as well as for concrete poured at the constructions site. Halfen channels are ideal, for framing applications, and for facade fixing solutions. A highly competent Research and Development team makes sure, that HALFEN-DEHA maintains its reputation as technology leader by offering the most innovative and the user-friendly fixing and anchoring systems.

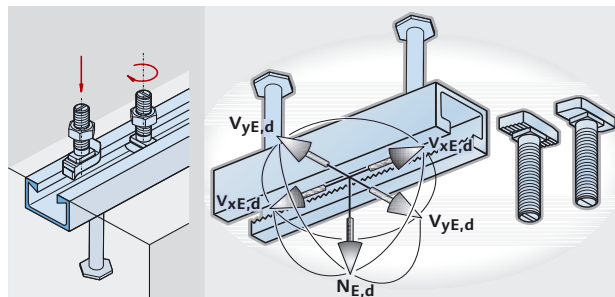


Oriental Plaza, Beijing      COFCO Plaza, Beijing



Tianan Plaza, Tianan      Hong Kong Chek Lap Kok Airport

### The Halfen System



New World Centre, Beijing

## INSTALLATION EXAMPLES

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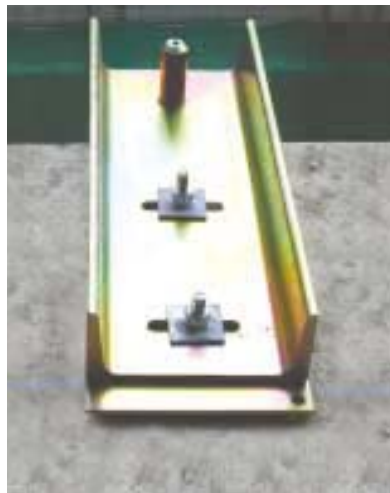
Typical facade connection to the top of floor slab.



Typical facade connection to the edge of a floor slab.



Facade connections using toothed HZA channels to provide adjustment in the direction of the wind load. One or two channels can be used per bracket.



Typical connection to the edge of a post tensioned floor slab.

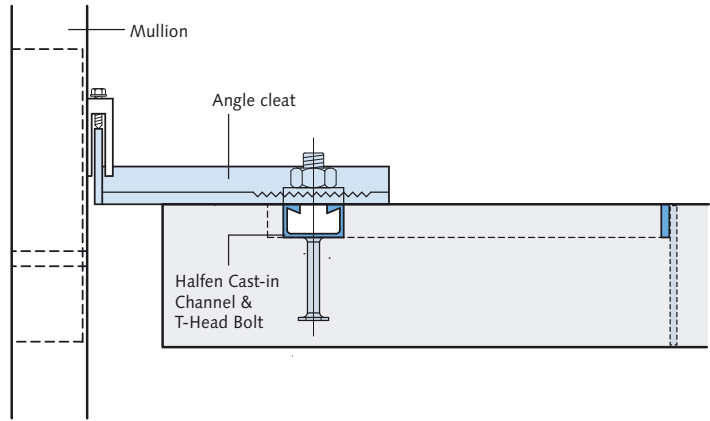
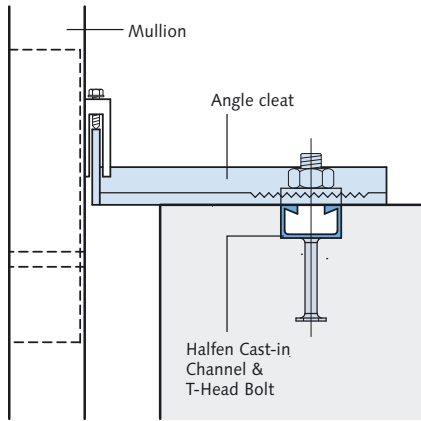
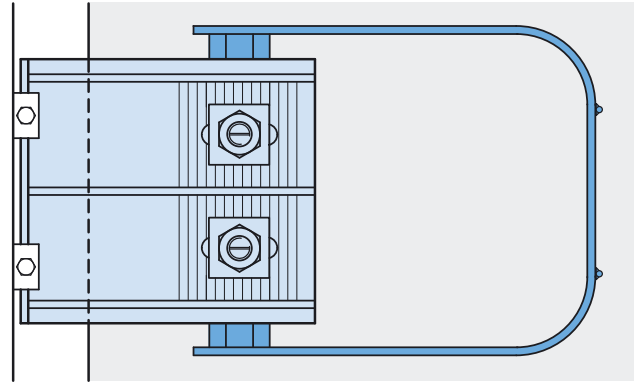
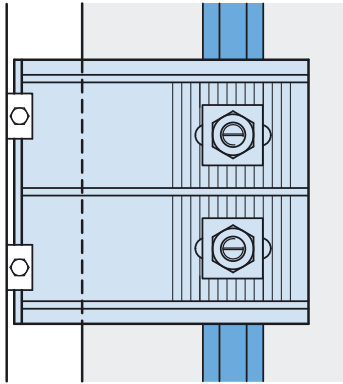


Sunscreen connections using channels to support vertical & horizontal loads.



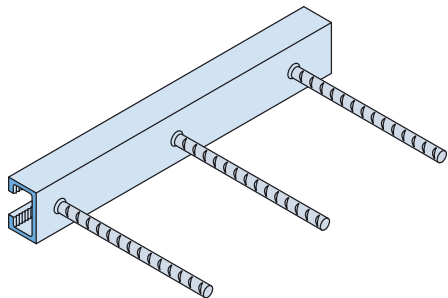
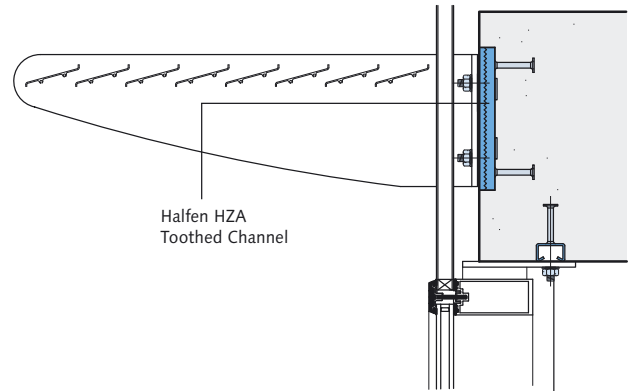
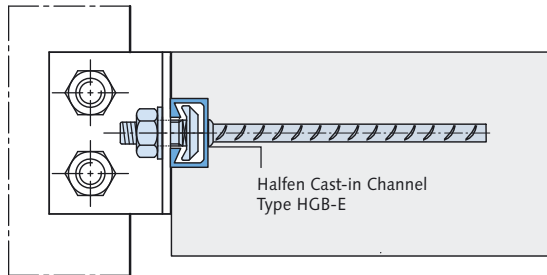
Typical window connection to precast panel.

# TYPICAL CURTAIN WALL CONNECTIONS



Typical anchorage detail to top of slab

High wind load connection using Halfen high load channel HCW S2/34



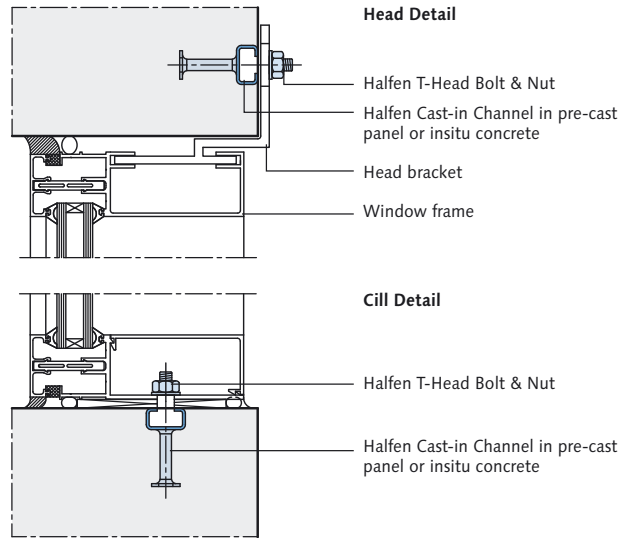
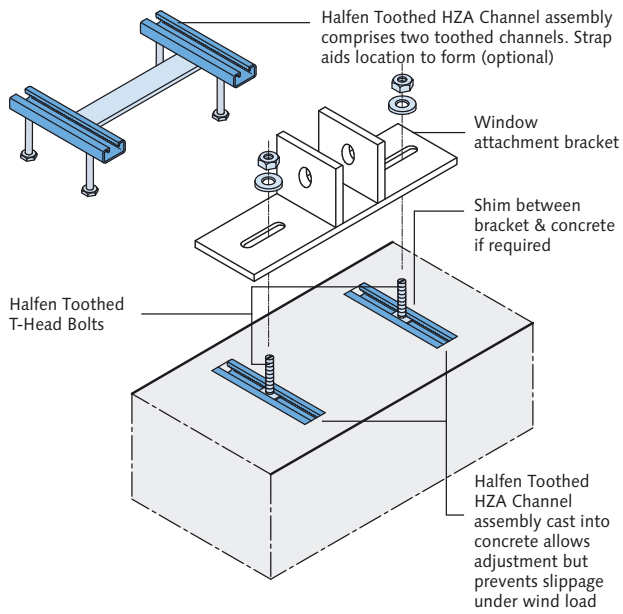
Curtain Walling bolted to structure using Halfen Cast-in Channel

Typical anchorage detail to edge of thin slab

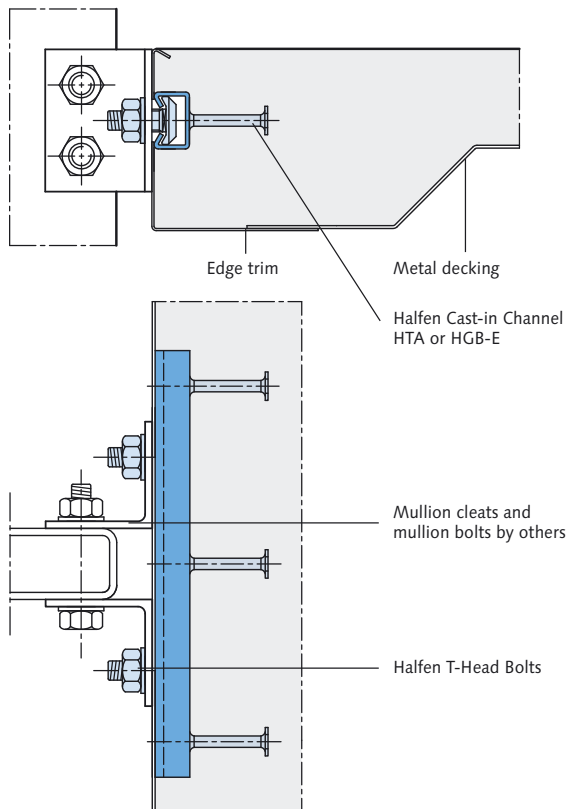
Typical sunscreen / glazing detail



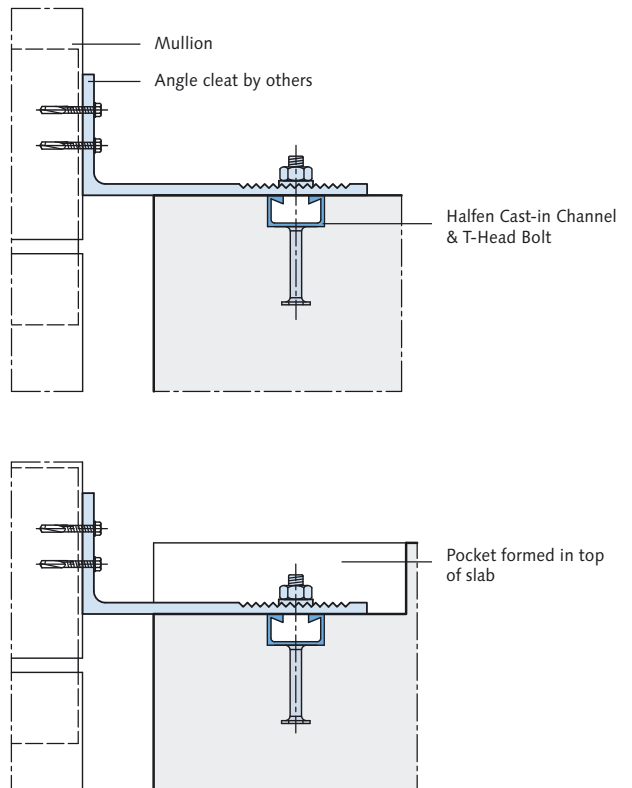
# TYPICAL CURTAIN WALL CONNECTIONS



Typical anchorage details for individual or strip windows



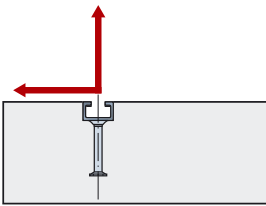
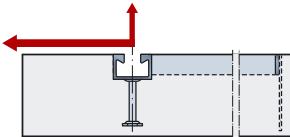
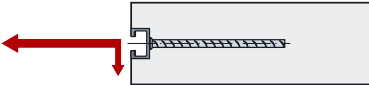
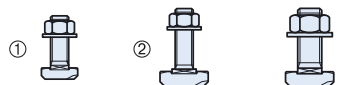
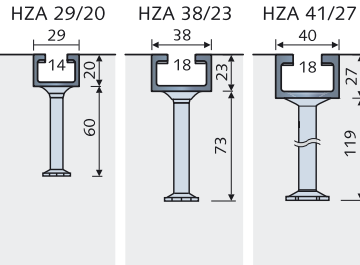
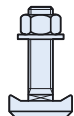
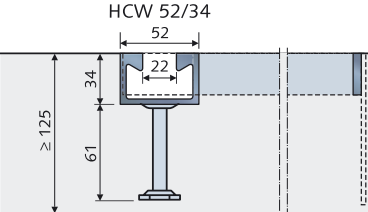

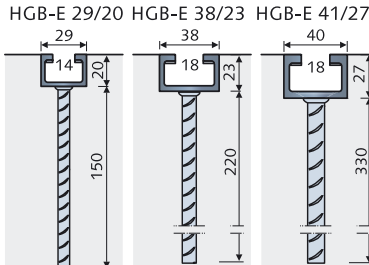

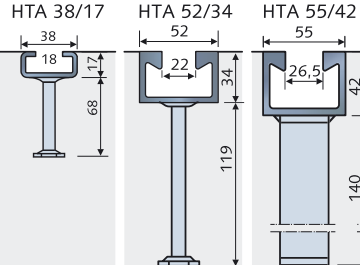
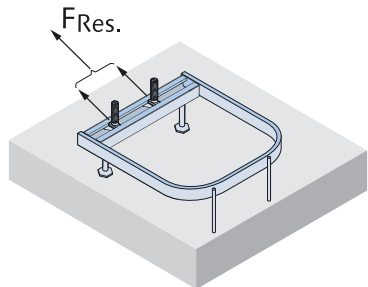

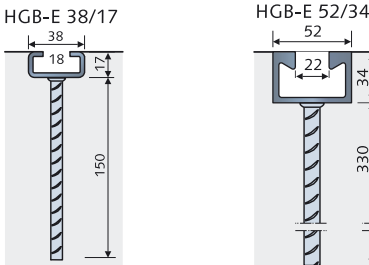
Typical anchorage detail to metal deck floor slab



Typical anchorage detail to top of floor slabs. Option of pocket at channel locations as illustrated.

# HALFEN CAST-IN CHANNELS HTA, HZA, HCW AND HGB-E

## SELECTING THE RIGHT HALFEN CHANNEL FOR EACH LOAD CONDITION

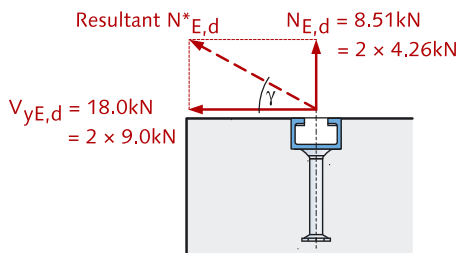
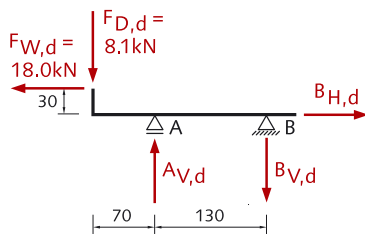
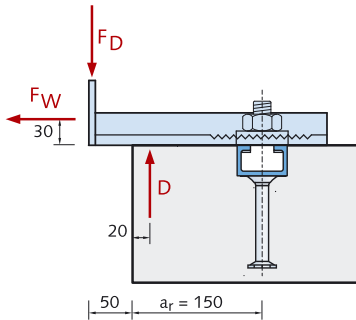
LOAD CONDITIONS AND REQUIRED HALFEN CHANNELS		
<p>Normal slab conditions → page 10 - 13</p> 	<p>Thin slab conditions with small edge distances High shear, medium tension → page 16 - 17</p> 	<p>Thin slab conditions High tension, small shear → page 14 - 15</p> 
<p><b>Standard Channels with Bolt Anchors</b></p>	<p><b>High Load Channel</b></p>	<p><b>Standard Channels with Bar Anchors</b></p>
<p>• Toothed channels + bolts</p> <p>HZS/HS 29/20 M10 / M12    HZS/HS 38/23 M12 / M16    HZS/HS 38/23 M16</p>  <p>HZA 29/20    HZA 38/23    HZA 41/27</p> 	<p>HS 50/30, M16, M20 grade 8.8</p>  <p>HCW 52/34</p> 	<p>• Toothed channels + bolts</p> <p>HZS/HS 29/20 M10 / M12    HZS/HS 38/23 M12 / M16    HZS/HS 38/23 M16</p>  <p>HGB-E 29/20    HGB-E 38/23    HGB-E 41/27</p> 
<p>• Plain channels + bolts</p> <p>HS 38/17 M12    HS 50/30, M16 HSR 50/30, M20</p>  <p>HTA 38/17    HTA 52/34    HTA 55/42</p> 		<p>• Plain channels + bolts</p> <p>HS 38/17 M12, M16    HS 50/30 M12, M16</p>  <p>HGB-E 38/17    HGB-E 52/34</p> 

**Notes:**

- ① HZA 29/20 replaces and offers improved performance to HTA 40/22.
- ② HZA 38/23 replaces and offers improved performance to HTA 50/30.

## CALCULATION EXAMPLE – HOW TO SELECT THE RIGHT CHANNEL

### EXAMPLE 1



#### Given:

working loads on the Curtain Wall bracket

- working deadload  $F_D = 6.00 \text{ kN}$
- working windload  $F_W = 12.00 \text{ kN}$

The calculation example uses load and resistance factor design, applying partial safety factors  $\gamma_F$  on the load side (action).

For deadloads:  $\gamma_F = 1.35$  (acc. to German regulations ①)

For windloads:  $\gamma_F = 1.50$  (acc. to German regulations ①)

⇒

- design deadload  $F_{D,d} = \gamma_F \cdot F_D = 1.35 \cdot 6.0 \text{ kN} = 8.1 \text{ kN}$

- design windload  $F_{W,d} = \gamma_F \cdot F_W = 1.5 \cdot 12.0 \text{ kN} = 18.00 \text{ kN}$

#### Design forces acting on the channel:

$$B_{V,d} = (8.1 \cdot 70 + 18.0 \cdot 30) / 130 = 8.51 \hat{=} N_{E,d}$$

$$B_{H,d} = F_{W,d} = 18.0 \text{ kN} \hat{=} V_{yE,d}$$

$$\gamma = \arctan\left(\frac{N_{E,d}}{V_{yE,d}}\right) = \arctan\left(\frac{8.51}{18.0}\right) = 25.3^\circ > 15^\circ$$

$$\Rightarrow N_{E,d}^* (\gamma > 15^\circ) = \sqrt{(N_{E,d})^2 + (V_{yE,d})^2}$$

$$N_{E,d}^* = \sqrt{(8.51)^2 + (18.0)^2} = 19.91 \text{ kN} \\ \hat{=} 2 \times 9.96 \text{ kN}$$

#### SELECTED CHANNEL:

**HZA 38/23 - 300 - 3 anchors with 2 bolts at 125mm centre** (→ p. 12)

required  $a_r = 150 \text{ mm}$  (→ p. 13) ✓ OK

$V_{yR,d} = 2 \times 9.0 \geq V_{yE,d} = 2 \times 9.0$  ✓ OK

$N_{R,d} = 2 \times 14.0 > N_{E,d} = 2 \times 4.26$  ✓ OK

$N_{R,d}^* = 2 \times 14.0 > N_{E,d}^* = 2 \times 9.96$  ✓ OK

#### SELECTED BOLTS:

**2 pieces HS 38/23 M12x60 gv 8.8** (→ p. 13)

$V_{yR,d} = 27.0 > V_{yE,d} = 9.0$  ✓ OK

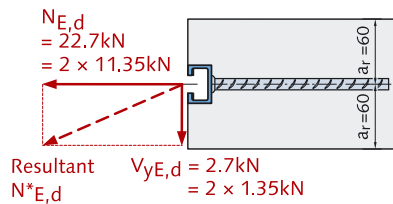
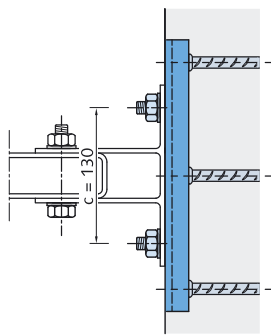
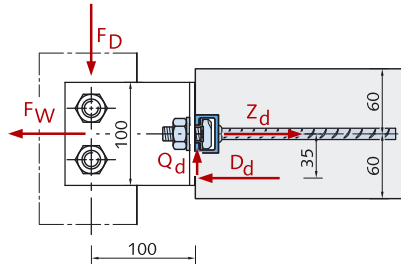
$N_{R,d} = 27.0 > N_{E,d} = 4.26$  ✓ OK

$FSR,d = 27.0 > FSE,d = 9.96$  ✓ OK

① Partial safety factors  $\gamma_F$  in China may be different from the German ones

## CALCULATION EXAMPLE – HOW TO SELECT THE RIGHT CHANNEL

### EXAMPLE 2



#### Given:

working loads on the Curtain Wall bracket

- working deadload  $F_D = 2.00 \text{ kN}$
- working windload  $F_W = 10.00 \text{ kN}$

The calculation example uses load and resistance factor design, applying partial safety factors  $\gamma_F$  on the load side (action).

For deadloads:  $\gamma_F = 1.35$  (acc. to German regulations ①)

For windloads:  $\gamma_F = 1.50$  (acc. to German regulations ①)

⇒

- design deadload  $F_{D,d} = \gamma_F \cdot F_D = 1.35 \cdot 2.0 \text{ kN} = 2.7 \text{ kN}$

- design windload  $F_{W,d} = \gamma_F \cdot F_W = 1.5 \cdot 10.0 \text{ kN} = 15.0 \text{ kN}$

#### Design forces acting on the channel:

$$Q_{,d} = F_{D,d} = 2.7 \text{ kN} \hat{=} V_{yE,d}$$

$$Z_{,d} = F_{W,d} + F_{D,d} \times (100 / 35) = 15.0 + 2.7 \times (100 / 35) = 22.7 \text{ kN} \hat{=} N_{E,d}$$

$$N^*_{E,d} = \sqrt{(N_{E,d})^2 + (V_{yE,d})^2}$$

$$N^*_{E,d} = \sqrt{(22.7)^2 + (2.7)^2} = 22.86 \text{ kN} = 2 \times 11.43 \text{ kN}$$

#### SELECTED CHANNEL:

**HGB-E 38/23 - 300 - 3 anchors with 2 bolts at 130mm centre** (→ p. 14)

actual  $a_r = 60 \text{ mm}$  (→ p. 14)

$$\Rightarrow V_{yR,d} = 2 \times 3.7 > V_{yE,d} = 2 \times 1.35 \quad \checkmark \text{ OK}$$

$$N_{R,d} = 2 \times 14.0 > N_{E,d} = 2 \times 11.35 \quad \checkmark \text{ OK}$$

$$N^*_{R,d} = 2 \times 14.0 > N^*_{E,d} = 2 \times 11.43 \quad \checkmark \text{ OK}$$

#### SELECTED BOLTS:

**2 pieces HS 38/23 M12×60 gv 8.8** (→ p. 15)

$$\text{actual } c = 130 > \text{required } c \geq 125 \text{ (→ p. 14)} \quad \checkmark \text{ OK}$$

$$V_{yR,d} = 27.0 > V_{yE,d} = 1.35 \quad \checkmark \text{ OK}$$

$$N_{R,d} = 27.0 > N_{E,d} = 11.35 \quad \checkmark \text{ OK}$$

$$F_{SR,d} = 27.0 > F_{SE,d} = 11.43 \quad \checkmark \text{ OK}$$

① Partial safety factors  $\gamma_F$  in China may be different from the German ones



# HALFEN CAST-IN CHANNELS

## INSTALLATION



Nail channel to formwork



When concrete is set, remove the formwork, then remove the filler material from the channel.



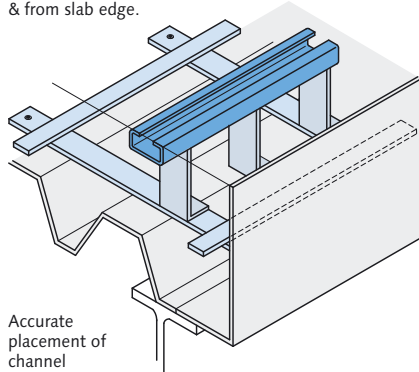
Insert T-bolt into channel and rotate ninety degrees.



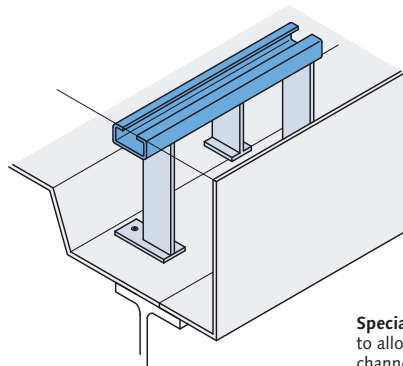
Attach component and tighten nut.

## SPECIAL INSTALLATION ASSEMBLIES

**Fabricated Ski Assembly**  
Factory fabricated assembly assures channel positioning on top of ribs & from slab edge.

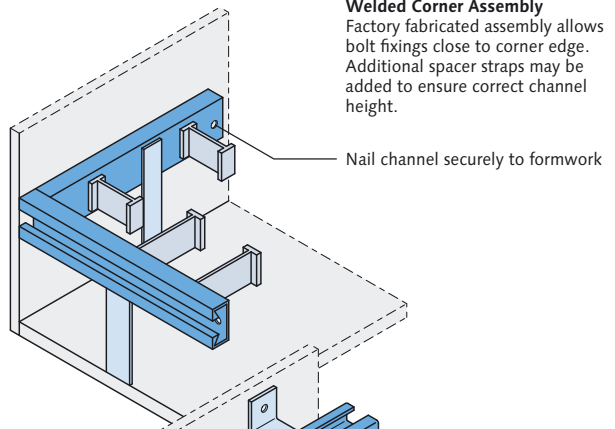


Accurate placement of channel

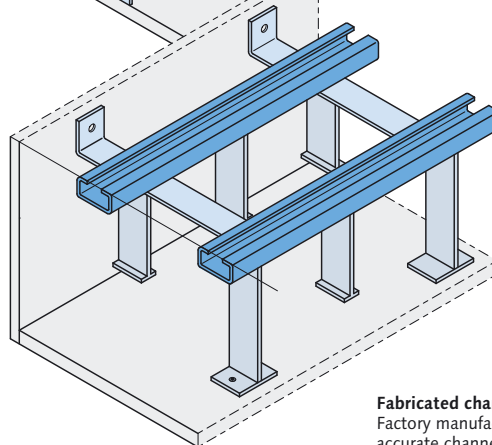


**Special Anchor Fabrications**  
to allow positioning and fixing of channel to troughs in metal deck.

**Welded Corner Assembly**  
Factory fabricated assembly allows bolt fixings close to corner edge. Additional spacer straps may be added to ensure correct channel height.



Nail channel securely to formwork



**Fabricated channel pair**  
Factory manufactured to provide accurate channel spacing.

# HALFEN CAST-IN CHANNELS AND T-HEAD BOLTS

## FOR EXTREME SITUATIONS

FACTORED DESIGN LOAD CONDITIONS					
<b>Structural analysis</b>		$F_{R,d}$ Material resistance	$F_{E,d}$ Factored design load		
1	Shear Material Resistance	$V_{yR,d}$	$\geq$	$V_{yE,d}$	
1	Tension Material Resistance	$N_{R,d}$	$\geq$	$N_{E,d}$	
2	Longitudinal Shear Material Resistance	$V_{xR,d}$	$\geq$	$V_{xE,d}$	
3	Resultant Shear Material Resistance	$V^*_{R,d}$	$\geq$	res. $V_{E,d}$	$= \sqrt{(V_{xE,d})^2 + (V_{yE,d})^2}$
		$V^*_{R,d}$	$\geq$	$V^*_{E,d}$ ( $\gamma \leq 15^\circ$ ①)	$= \sqrt{(N_{E,d})^2 + (\text{res. } V_{E,d})^2}$
4	Resultant Tension Material Resistance	$N^*_{R,d}$	$\geq$	$N^*_{E,d}$ ( $\gamma \geq 15^\circ, \alpha \leq 150^\circ$ ①)	$= \sqrt{(N_{E,d})^2 + (\text{res. } V_{E,d})^2}$
5	Dynamic Material Resistance	$\Delta N_{R,d}$	$\geq$	$\Delta N_{E,d}$	
				① $\gamma = \arctan\left(\frac{N_{E,d}}{\text{res. } V_{E,d}}\right)$	

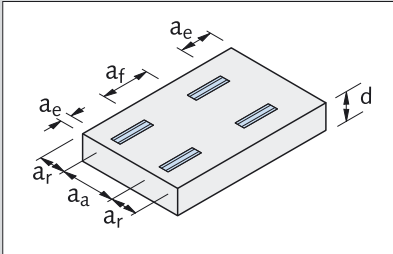
HALFEN CAST-IN CHANNELS HTA AND HZA – MATERIAL DESIGN RESISTANCE VALUES					
Channel Type:	HTA 52/34 350 mm 3 anchors	HTA 55/42 350 mm 3 anchors	HZA 29/20 250 mm 2 anchors	HZA 38/23 300 mm 3 anchors	HZA 41/27 350 mm 3 anchors
Concrete compression strength $\geq$ C20/25 $f_{CK, cyl.} = 20 \text{ N/mm}^2$ $f_{CK, cube} = 25 \text{ N/mm}^2$					
$N_{R,d}$ [kN]	2 x 30	2 x 37	2 x 8,5	2 x 14	2 x 28
$V_{yR,d}$ [kN]	2 x 15	2 x 18	2 x 8,5	2 x 9	2 x 15
$V_{xR,d}$ [kN]	2 x 10 ②	2 x 10 ②	2 x 7,5	2 x 11	2 x 12
$N^*_{R,d}$ [kN]	2 x 30	2 x 37	2 x 8,5	2 x 14	2 x 28
$V^*_{R,d}$ [kN]	2 x 15	2 x 18	2 x 8,5	2 x 9	2 x 15
$\Delta N_{R,d}$ [kN]	2 x 9 ③	2 x 10 ③	2 x 2,5	2 x 4	2 x 4
Material: hot-dip galvanised	channel: W1.0038 anchor: W1.5523 or W1.5535	channel: W1.0044 anchor: W1.0038	channel: W1.0044 anchor: W1.0205	channel: W1.0044 anchor: W1.0205	channel: W1.0044 anchor: W1.5523 or W1.5535
Notes: ② only with HSR M20 ③ only with welded I-anchor Q					

# HALFEN CAST-IN CHANNELS AND T-HEAD BOLTS

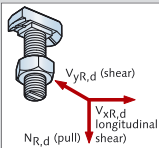
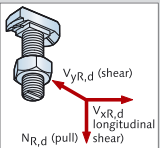

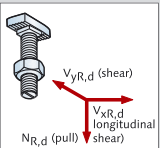
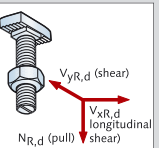
## FOR EXTREME SITUATIONS

### HALFEN CAST-IN CHANNELS HTA/HZA – MINIMUM SPACING $a_r$ , $a_a$ , $a_e$ , $a_f$ AND $d$

Minimum spacings and edge distances [mm] ④ For all concrete grades $\geq$ C20/25					
Halfen Channel Type	$a_r$	$a_a$	$a_e$	$a_f$	$d$ ⑤
HTA 52/34	200	400	175	350	160 + nom.c
HTA 55/42	250	500	225	450	185 + nom.c
HZA 29/20	100	200	80	200	80 + nom.c
HZA 38/23	150	300	130	250	96 + nom.c
HZA 41/27	200	400	175	350	146 + nom.c

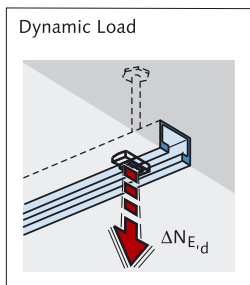
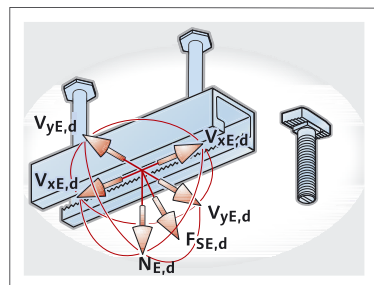


### HALFEN T-HEAD BOLTS HSR, HZS – MATERIAL DESIGN RESISTANCE PER BOLT ⑥

Halfen Channel Type	HTA 52/34 350 mm 3 anchors	HTA 55/42 350 mm 3 anchors	HZA 29/20 250 mm 2 anchors	HZA 38/23 300 mm 3 anchors	HZA 41/27 350 mm 3 anchors
Halfen T-head bolt Type	HSR 50/30 M20x60 gv 8.8	HSR 50/30 M20x60 gv 8.8	HZS 29/20 M12x60 gv 8.8	HZS 38/23 M12x60 gv 8.8 (HZS 38/23 M16x60 gv 8.8)	HZS 38/23 M16x60 gv 8.8
					
$N_{R,d} = V_{yR,d} = F_{SR,d}$ [kN]	78	78	27	27 (50)	50
$V_{xR,d}$ [kN]	10	10	27	27 (50)	50
$\Delta N_{R,d}$ [kN]	10	10	2	2 (4)	4
Required tightening torque [Nm]	400	400	80	80 (120)	120

#### Structural analysis

	$F_{R,d}$ Material resistance	$F_{E,d}$ Factored design load
Shear Material Resistance	$V_{yR,d}$	$V_{yE,d}$
Tension Material Resistance	$N_{R,d}$	$N_{E,d}$
Longitudinal Shear Material Resistance	$V_{xR,d}$	$V_{xE,d}$
Resultant Material Resistance	$F_{SR,d}$	$F_{SE,d} = \sqrt{(N_{E,d})^2 + (V_{xE,d})^2 + (V_{yE,d})^2}$
Dynamic Material Resistance	$\Delta N_{R,d}$	$\Delta N_{E,d}$



#### Notes:

- ④ The minimum dimensions given in the table apply to reinforced concrete. For unreinforced concrete increase dimensions by 30%.
- ⑤ Derived from channel plus anchor plus the required concrete cover (e.g. to DIN 1045).
- ⑥ Channel load capacity must not be exceeded!  
gv 4.6 = zinc plated, strength grade 4.6  
gv 8.8 = zinc plated, strength grade 8.8

# HALFEN CAST-IN CHANNELS AND T-HEAD BOLTS

## FOR STANDARD APPLICATIONS

**FACTORED DESIGN LOAD CONDITIONS**

**Structural analysis**

	$F_{R,d}$ Material resistance	$F_{E,d}$ Factored design load
1 Shear Material Resistance	$V_{yR,d} \geq$	$V_{yE,d}$
1 Tension Material Resistance	$N_{R,d} \geq$	$N_{E,d}$
3 Resultant Shear Material Resistance	$V^*_{R,d} \geq$	$V^*_{E,d} \ (\gamma \leq 15^\circ \text{ ①}) = \sqrt{(N_{E,d})^2 + (V_{yE,d})^2}$
4 Resultant Tension Material Resistance	$N^*_{R,d} \geq$	$N^*_{E,d} \ (\gamma \geq 15^\circ, \alpha \leq 150^\circ \text{ ①}) = \sqrt{(N_{E,d})^2 + (V_{yE,d})^2}$

①  $\gamma = \arctan\left(\frac{N_{E,d}}{V_{yE,d}}\right)$

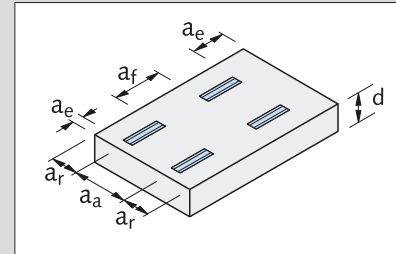
**HALFEN CAST-IN CHANNELS HTA AND HZA – MATERIAL DESIGN RESISTANCE VALUES**

Channel Type:	HTA 38/17 250 mm 2 anchors	HTA 52/34 350 mm 3 anchors	HTA 55/42 350 mm 3 anchors	HZA 29/20 250 mm 2 anchors	HZA 38/23 300 mm 3 anchors	HZA 41/27 350 mm 3 anchors
Concrete compression strength $\geq C20/25$ $f_{CK, cyl.} = 20 \text{ N/mm}^2$ $f_{CK, cube} = 25 \text{ N/mm}^2$						
$N_{R,d}$ [kN]	2 x 6	2 x 30	2 x 37	2 x 8,5	2 x 14	2 x 28
$V_{yR,d}$ [kN]	2 x 6	2 x 15	2 x 18	2 x 8,5	2 x 9	2 x 15
$N^*_{R,d}$ [kN]	2 x 6	2 x 30	2 x 37	2 x 8,5	2 x 14	2 x 28
$V^*_{R,d}$ [kN]	2 x 6	2 x 15	2 x 18	2 x 8,5	2 x 9	2 x 15
Material: hot-dip galvanised	channel: W1.0037 anchor: W1.0205	channel: W1.0038 anchor: W1.5523 or W1.5535	channel: W1.0044 anchor: W1.0038	channel: W1.0044 anchor: W1.0205	channel: W1.0044 anchor: W1.0205	channel: W1.0044 anchor: W1.5523 or W1.5535

# HALFEN CAST-IN CHANNELS AND T-HEAD BOLTS

## FOR STANDARD APPLICATIONS

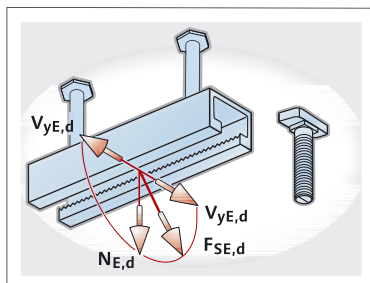
HALFEN CAST-IN CHANNELS HTA/HZA – MINIMUM SPACING $a_r$ , $a_a$ , $a_e$ , $a_f$ AND $d$					
Minimum spacings and edge distances [mm] ② For all concrete grades $\geq$ C20/25					
Halfen Channel Type	$a_r$	$a_a$	$a_e$	$a_f$	$d$ ③
HTA 38/17	75	150	50	100	70 + nom.c
HTA 52/34	200	400	175	350	160 + nom.c
HTA 55/42	250	500	225	450	185 + nom.c
HZA 29/20	100	200	80	200	80 + nom.c
HZA 38/23	150	300	130	250	96 + nom.c
HZA 41/27	200	400	175	350	146 + nom.c



HALFEN T-HEAD BOLTS HS – MATERIAL DESIGN RESISTANCE PER BOLT ④						
Halfen Channel Type	HTA 38/17 250 mm 2 anchors	HTA 52/34 350 mm 3 anchors	HTA 55/42 350 mm 3 anchors	HZA 29/20 250 mm 2 anchors	HZA 38/23 300 mm 3 anchors	HZA 41/27 350 mm 3 anchors
Halfen T-head bolt Type	HS 38/17 M12×60 gv 4.6	HS 50/30 M16×60 gv 8.8	HS 50/30 M16×60 gv 8.8	HS 29/20 M10×60 gv 8.8	HS 38/23 M12×60 gv 8.8 HS 38/23 (M16×60 gv 4.6)	HS 38/23 M16×60 gv 8.8
$N_{R,d} = V_{yR,d} = F_{SR,d}$ [kN]	13	50	50	18	27 (24)	50
Required tightening torque [Nm]	25	200	200	48	70 (60)	200

### Structural analysis

	$F_{R,d}$	$F_{E,d}$	
	Material <u>resistance</u>	Factored design <u>load</u>	
Shear Material Resistance	$V_{yR,d}$	$\geq$	$V_{yE,d}$
Tension Material Resistance	$N_{R,d}$	$\geq$	$N_{E,d}$
Resultant Material Resistance	$F_{SR,d}$	$\geq$	$F_{SE,d} = \sqrt{(N_{E,d})^2 + (V_{yE,d})^2}$



### Notes:

- ② The minimum dimensions given in the table apply to reinforced concrete. For unreinforced concrete increase dimensions by 30%.
- ③ Derived from channel plus anchor plus the required concrete cover (e.g. to DIN 1045).
- ④ Channel load capacity must not be exceeded!  
gv 4.6 = zinc plated, strength grade 4.6  
gv 8.8 = zinc plated, strength grade 8.8



# HALFEN CAST-IN CHANNELS AND T-HEAD BOLTS

## FOR USE IN THIN CONCRETE ELEMENTS

**FACTORED DESIGN LOAD CONDITIONS**

Shear ( $V_{yE,d}$ )  
Tension ( $N_{E,d}$ )  
res. Tension ( $N^*_{E,d}$ )

Minimum spacing  $a_r, a_e$

**Structural analysis**

	$F_{R,d}$ Material resistance	$F_{E,d}$ Factored design load	
<span style="border: 1px solid black; padding: 2px;">7</span> Shear Material Resistance	$V_{yR,d}$	$V_{yE,d}$	$\geq$
<span style="border: 1px solid black; padding: 2px;">7</span> Tension Material Resistance	$N_{R,d}$	$N_{E,d}$	$\geq$
<span style="border: 1px solid black; padding: 2px;">7</span> Resultant Tension Material Resistance	$N^*_{R,d}$	$N^*_{E,d}$	$\geq$

$$= \sqrt{(N_{E,d})^2 + (V_{yE,d})^2}$$

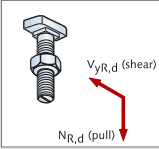
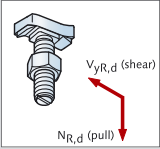
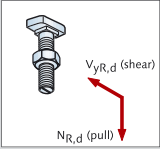
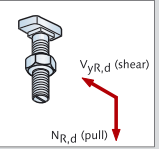
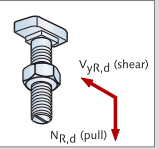
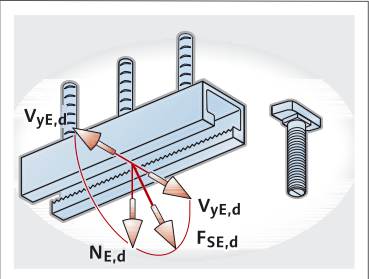
Notes:  
① The minimum dimensions given in the table apply to reinforced concrete.

**HALFEN CAST-IN CHANNELS HTA AND HZA – MATERIAL DESIGN RESISTANCE VALUES**

Channel Type:	HGB-E 38/17	HGB-E 52/34	HGB-E 29/20	HGB-E 38/23	HGB-E 41/27
Concrete compression strength $\geq$ C20/25 $f_{CK, cyl.} = 20 \text{ N/mm}^2$ $f_{CK, cube} = 25 \text{ N/mm}^2$					
$N_{R,d} = N^*_{R,d}$ [kN]	2 x 7	2 x 24	2 x 9	2 x 14	2 x 22
$a_r$ [mm]   $a_e$ [mm]	$V_{yR,d}$ [kN]				
$\geq 50$   $\geq 40$	2 x 2.4				
$\geq 60$   $\geq 45$	2 x 3.7				
$\geq 70$   $\geq 50$	2 x 4.9				
$\geq 75$   $\geq 50$	2 x 5.6				
Material: hot-dip galvanised	channel: W1.0037 anchor: BSt 5005	channel: W1.0038 anchor: BSt 5005	channel: W1.0044 anchor: BSt 5005	channel: W1.0044 anchor: BSt 5005	channel: W1.0044 anchor: BSt 5005

# HALFEN CAST-IN CHANNELS AND T-HEAD BOLTS

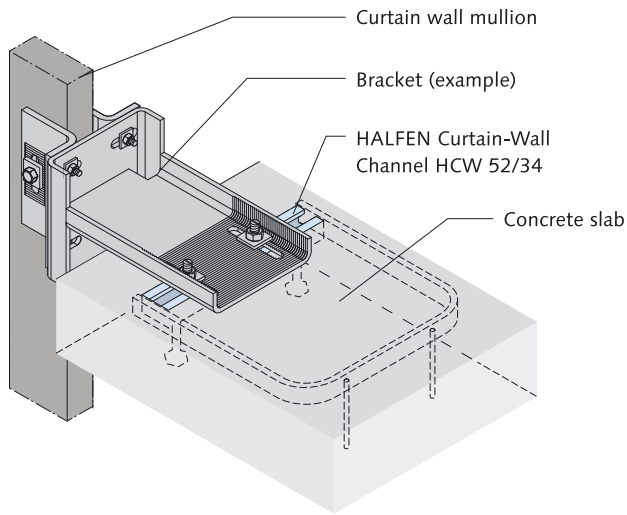
## FOR USE IN THIN CONCRETE ELEMENTS

HALFEN T-HEAD BOLTS HS – MATERIAL DESIGN RESISTANCE PER BOLT ②					
Halfen Channel Type	HGB-E 38/17 300 mm 3 anchors	HGB-E 52/34 300 mm 3 anchors	HGB-E 29/20 300 mm 3 anchors	HGB-E 38/23 300 mm 3 anchors	HGB-E 41/27 300 mm 3 anchors
Halfen T-head bolt Type	HS 38/17 M12×60 gv 4.6	HS 50/30 M16×60 gv 8.8	HS 29/20 M10×60 gv 8.8	HS 38/23 M12×60 gv 8.8 (HS 38/23 M16×60 gv 4.6)	HS 38/23 M16×60 gv 8.8
					
$N_{R,d} = V_{yR,d} = F_{SR,d}$ [kN]	13	50	18	27 (24)	50
Required tightening torque [Nm]	25	60	40	40 (60)	60
<b>Structural analysis</b>	$F_{R,d}$ Material resistance		$F_{E,d}$ Factored design load		
Shear Material Resistance	$V_{yR,d}$	≥	$V_{yE,d}$		
Tension Material Resistance	$N_{R,d}$	≥	$N_{E,d}$		
Resultant Material Resistance	$F_{SR,d}$	≥	$F_{SE,d}$	$= \sqrt{(N_{E,d})^2 + (V_{yE,d})^2}$	
			<p>Notes:</p> <p>② Channel load capacity must not be exceeded!</p> <p>gv 4.6 = zinc plated, strength grade 4.6</p> <p>gv 8.8 = zinc plated, strength grade 8.8</p>		

# HALFEN HIGH LOAD CHANNEL HCW 52/34

## FOR CURTAIN WALL CONNECTIONS

### TYPICAL INSTALLATION

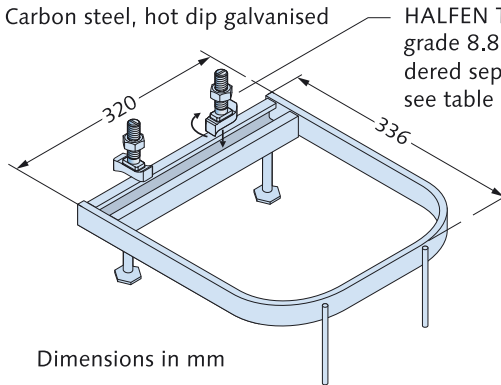


### PRODUCT DESCRIPTION

Product Code:  
HCW 52/34

Material:  
Carbon steel, hot dip galvanised

HALFEN T-head bolts  
grade 8.8 (to be ordered separately → see table page 17)

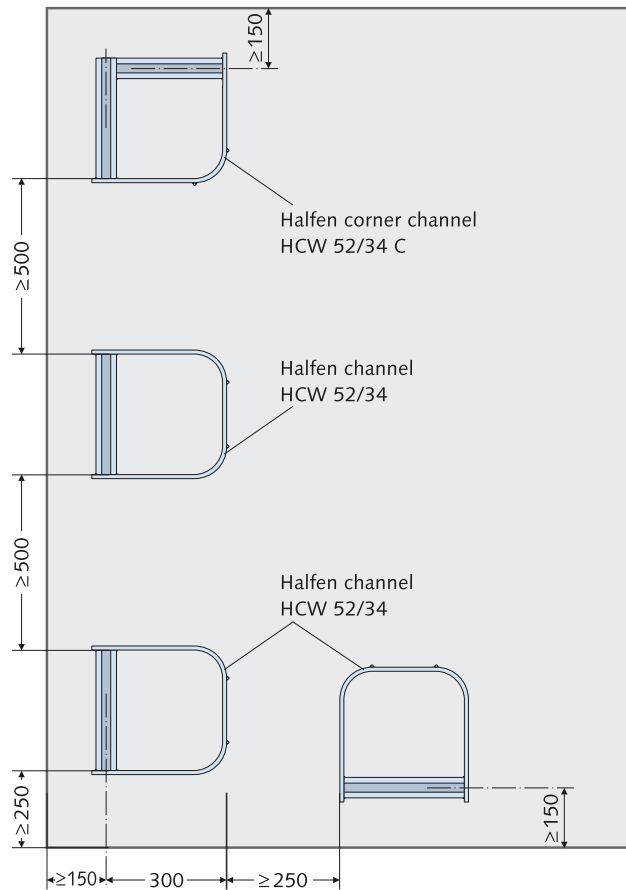
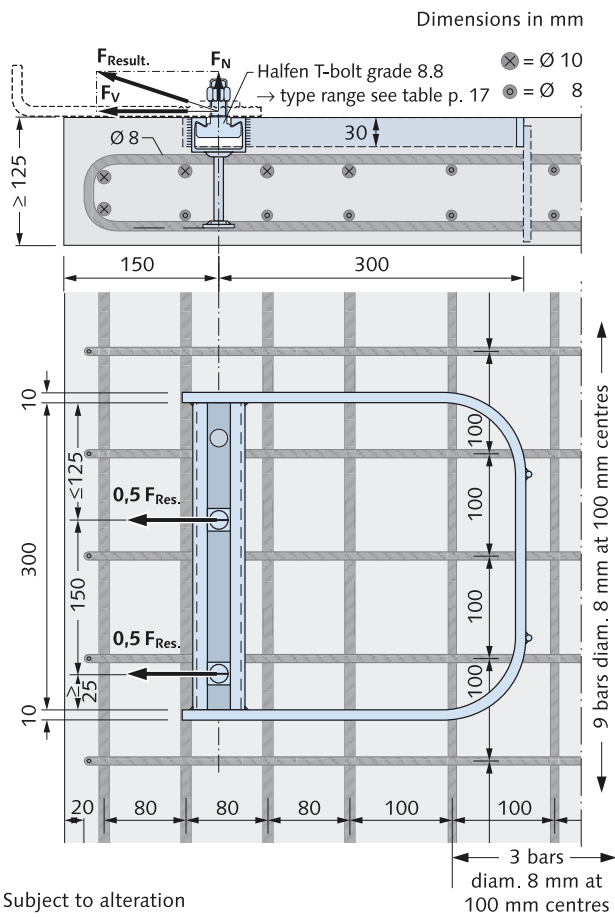


Dimensions in mm

### CHANNEL DIMENSIONS AND POSITIONING

Halfen channel HCW 52/34 should be placed in the concrete according to the minimum edge distances and spacing dimensions shown in the drawing below. Corner conditions using Halfen channel HCW 52/34 C will require special design, please contact us for assistance.

### REINFORCEMENT REQUIREMENTS



# HALFEN HIGH LOAD CHANNEL HCW 52/34

## FOR CURTAIN WALL CONNECTIONS

### CHANNEL LOAD DATA

A series of 3 tests produced the following average ultimate loads:

$F_{V \text{ ultimate}}$	= 142.3 kN
$F_{N \text{ ultimate}}$	= 47.4 kN
$F_{\text{Result. ultimate}} = \sqrt{F_N^2 + F_V^2}$	= 150.0 kN

The adjacent load deformation diagram may be used to determine allowable loads based on acceptable displacement and the required safety factor according to local building codes. The diagram is based on the following:

The concrete slab is  $\geq 125$  mm thick and reinforced according to the diagram on the previous page.

Concrete compression strength  $\geq C 20/25$  N/mm<sup>2</sup> (cylinder/cube) with normal weight aggregate.

Load is equally distributed to the channel by two Halfen T-bolts (ordered separately) spaced at  $\geq 150$  mm centers. See below for sizes and load capacities.

An example of a typical calculation method is shown below. The factors used in the calculation example are for illustration only. Actual factors used on a project basis must be checked according to local or national building regulations. Also the calculations make no allowance for load magnification due to load eccentricities. These must be included according to the project design of the connection. Contact us for guidance if required.

Calculation Example: Assumed safety factor 3 applied to the ultimate test load.

Ultimate test load:  $F_{\text{Result. ultimate}} = 150.0$  kN  
 $\Rightarrow F_{V \text{ ultimate}} = 142.3$  kN  
 $\Rightarrow F_{N \text{ ultimate}} = 47.4$  kN

Required working loads:  $F_{V \text{ work.}} = 35$  kN,  $F_{N \text{ work.}} = 10$  kN

Allowable load at 3:1 safety factor:

$F_{\text{Result. allow.}} = 50.0$  kN  
 $\Rightarrow F_{V \text{ allow.}} = 47.4$  kN  
 $\Rightarrow F_{N \text{ allow.}} = 15.8$  kN

Checking  $F_{V \text{ work.}} = 35$  kN < 47.4 kN → so OK

Checking  $F_{N \text{ work.}} = 10$  kN < 15.8 kN → so OK

Checking  $F_{\text{Result. work.}} = \sqrt{10^2 + 35^2} = 36.4$  kN < 50 kN → so OK

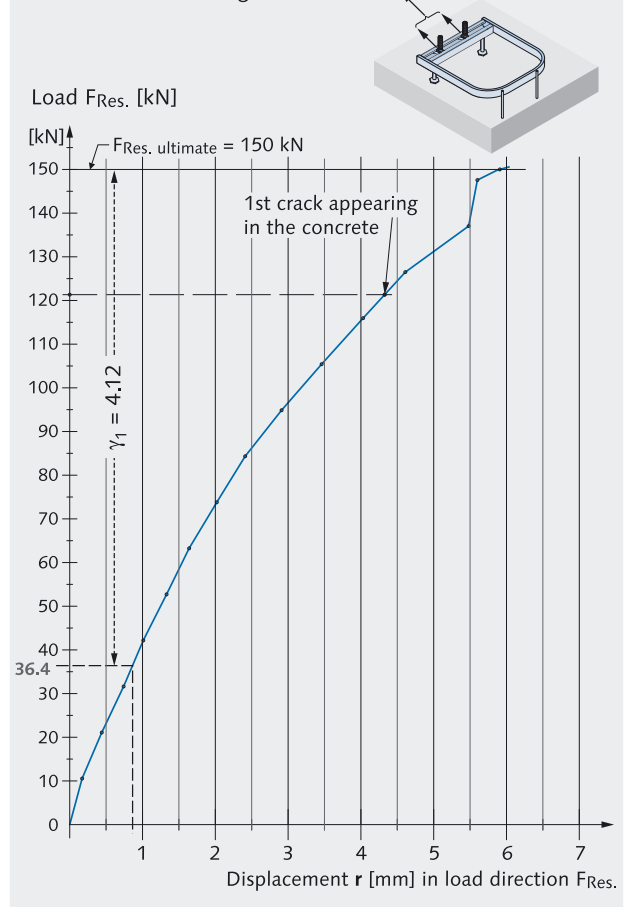
Displacement at working load < 1mm (see diagram).

Actual safety factor to ultimate test load:  $\gamma_1 = \frac{150}{36.4} = 4.12$

The load deformation diagram may also be used as a basis for calculations according to load resistance design methods.

Partial safety factors should be incorporated according to local or national design norms.

Load deformation diagram



### FASTENER INFORMATION

Halfen 8.8 grade T-bolts type HS 50/30, M16 and M20 are recommended for use with Halfen channel type HCW 52/34 according to the load performance required.

The loads  $F_{S \text{ Allow.}}$  shown in the table below are per bolt and based on applied safety factors of approximately 2.5:1,

Type selection Halfen 8.8 grade T-bolts HS 50/30

Thread Size	Material Grade DIN	L Available Lengths [mm]	$F_{S \text{ Allow.}}$ bolt load (pull, angled pull and shear) [kN]	Allowable Bending Moment [Nm]	Recommended initial torque [Nm]	If slotted holes are used in the bracket to achieve tolerance transverse to the channel, the capacity of the T-bolts should be checked according to the allowable bending moment.
M 16	8.8	40, 60, 80, 100	36.1	111	200	
M 20	8.8	45, 60, 80, 100	56.4	216	400	

other factors may be applied according to appropriate regulations and project requirements.

Please note, that fastener performance may be limited by channel capacity. The sizes shown are produced with a special coating equivalent to hot dip galvanising in salt spray tests. T-bolts in other sizes and materials are available if required, please contact us for details.

## CURTAIN WALL FIXING *WITHOUT* HALFEN CHANNELS: THE DISADVANTAGES



Welding is slow, is a fire risk, and needs to be closely checked for quality.



Weld and paint splatter can cause expensive damage to the facade.



Welded connections require painting to give minimal corrosion protection.

### WELDING

- Risk of sparks starting fires and damaging glass & aluminium facade = high risk & high cost.
- Quality welding difficult to achieve and check on site = high risk.
- Welding needs a lot of time and should be tested to verify quality = slow installation & high risk.
- Painting required after welding to provide any corrosion protection = poor corrosion protection, more time, another operation to check, dripping paint damages facade and creates health hazard.
- Heavy electrical equipment, trailing wires, and electricity = safety hazards.
- Welds must be broken and re-welded if adjustment needed after initial installation = slow installation.
- Embedded plates are designed per project and need testing to verify performance.



Welding requires difficult repositioning of heavy equipment and a costly energy supply.



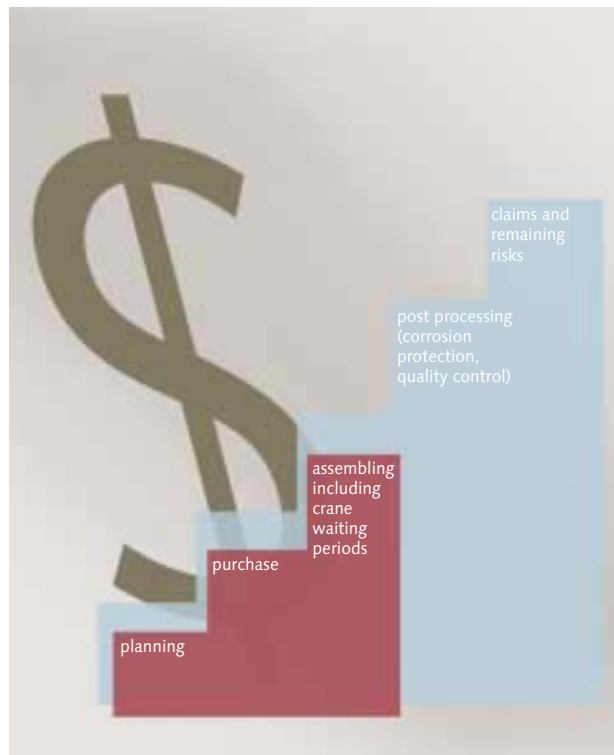
## CURTAIN WALL FIXING WITH HALFEN CHANNELS: THE ADVANTAGES

### HALFEN CHANNEL

- Only simple tools needed for installation.
- Easy and fast to install without special training.
- Components protected from corrosion by quality galvanising and plating.
- Can be installed without power supply.
- Easily adjustable connections.
- Fully tested components with verified load capacities.



### TOTAL COST COUNTS



Cast-In Channels

Welding Plates



The only tool required.